

Delta T_c (ΔT_c) Asphalt Binder Specification Parameter



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Presentation Overview

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- I. Introduction**
- II. Background**
- III. Determination of ΔT_c**
- IV. Elements Impacting ΔT_c**
- V. Steps to Implementation of ΔT_c**
- VI. Alternatives to ΔT_c and Ongoing Research**
- VII. Status of Implementation of ΔT_c**
- VIII. Summary**

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Introduction (1 of 4)

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- **Objective:** provide knowledge and technical support for responsible deployment of Delta T_c (ΔT_c) as a specification parameter into asphalt binder acceptance specifications.
- **Purpose:** provide preliminary considerations, if a State DOT has pressing needs and wants to proceed with implementation while acknowledging that information on ΔT_c continues to evolve.
- **Federal Highway Administration (FHWA) project:**
“Deployment and Development of Innovative Asphalt Pavement Technologies. (DDIAPT)”
 - ✦ Tech Brief: Delta T_c Binder Specification Parameter
https://www.fhwa.dot.gov/pavement/asphalt/HIF_Delta_Binder_Spec_TchBrf.pdf

Introduction (2 of 4)

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- **Delta T_c (ΔT_c) – “Calculated” Asphalt Binder Parameter**
- Provides Insight Into Relaxation Properties of Asphalt Binders
 - ✦ Non-Load Related Cracking
 - ✦ Other Age-Related Embrittlement Distresses in Asphalt Pavements
- **ΔT_c - Calculated using Bending Beam Rheometer (BBR)**
- Results**
- Long-Term Aged binder (rolling thin-film oven (RTFO) plus Pressure Aging Vessel (PAV))
- Recovered Binder from Recycled Asphalt Mixtures (RAP) and Recycled Asphalt Shingles (RAS)

Introduction (3 of 4)

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- **Most any type Asphalt Binder can be Evaluated with ΔT_c**
 - Neat Asphalt Binder (asphalt binder with no additives or modifiers), Extracted Binders
 - Asphalt Binders with additives (Anti-Strip, PPA, REOB/VTAE, and Warm-Mix Additives)
 - Modified Asphalt Binders with Polymers or other asphalt additives, (RAP, RAS, or Combinations of RAP and RAS)
- **ΔT_c May Indicate:**
 - Effectiveness of asphalt binder response to aging
 - Effectiveness of additive impact on response of asphalt binder to aging
- **State Departments of Transportation (DOTs) are currently Implementing or Considering Implementation of ΔT_c into Existing Acceptance Specifications**

Introduction (4 of 4)

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- **National Level Research Projects are currently Considering ΔT_c in Research**
- **Objective – Promote the “State-of-the-Knowledge” of ΔT_c as a Parameter to Characterize Asphalt Binder Behavior and aid in Affective Deployment as a Specification Parameter**
- **Excerpt and Summary from Asphalt Institute (AI) “State-of-the-Knowledge” Informational Series (IS) 240**

“Use of the Delta Tc Parameter to Characterize Asphalt Binder Behavior” (asphaltinstitute.org)

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- **The ΔT_c Parameter Conceptualized during SHRP and later suggested as a performance indicator in the Airfield Asphalt Pavement Technology Program (AAPTP), Project 06-01.**
 - Identify simple asphalt binder and/or asphalt mixture testing to predict imminent durability issues (cracking or raveling).
 - Facilitate timing of asphalt pavement preservation strategies.
- **AAPTP Concluded that ΔT_c could be used as a tool to Predict Ductility and Analyze Durability-Related Properties of Aged Asphalt Pavements.**
- **ΔT_c has Evolved as an Asphalt Binder Parameter that can be used to Evaluate Relaxation Properties of Asphalt Binders.**

Block Cracking

Relaxation Properties of aged Asphalt Binders, Expressed by ΔT_c Values, can Affect Different types of Asphalt Pavement Distresses:

- **Non-load related cracking**
- **Other age-related embrittlement distresses**
- **Only block cracking is affected directly**



Other Cracking Types

Other types of cracking are indirectly affected by ΔT_c :

- Fatigue
- Edge
- Longitudinal
- Reflective
- Transverse

While ΔT_c may be a contributing factor these types of cracking are predominately caused by other factors

Common Pavement Distresses	Effect of ΔT_c
Block Cracking	Direct
Fatigue Cracking	Indirect
Edge Cracking	Indirect
Longitudinal Cracking	Indirect
Reflection Cracking	Indirect
Transverse Cracking	Indirect
Potholes	Indirect
Raveling	Indirect
Rutting	None
Shoving	None
Bleeding	None

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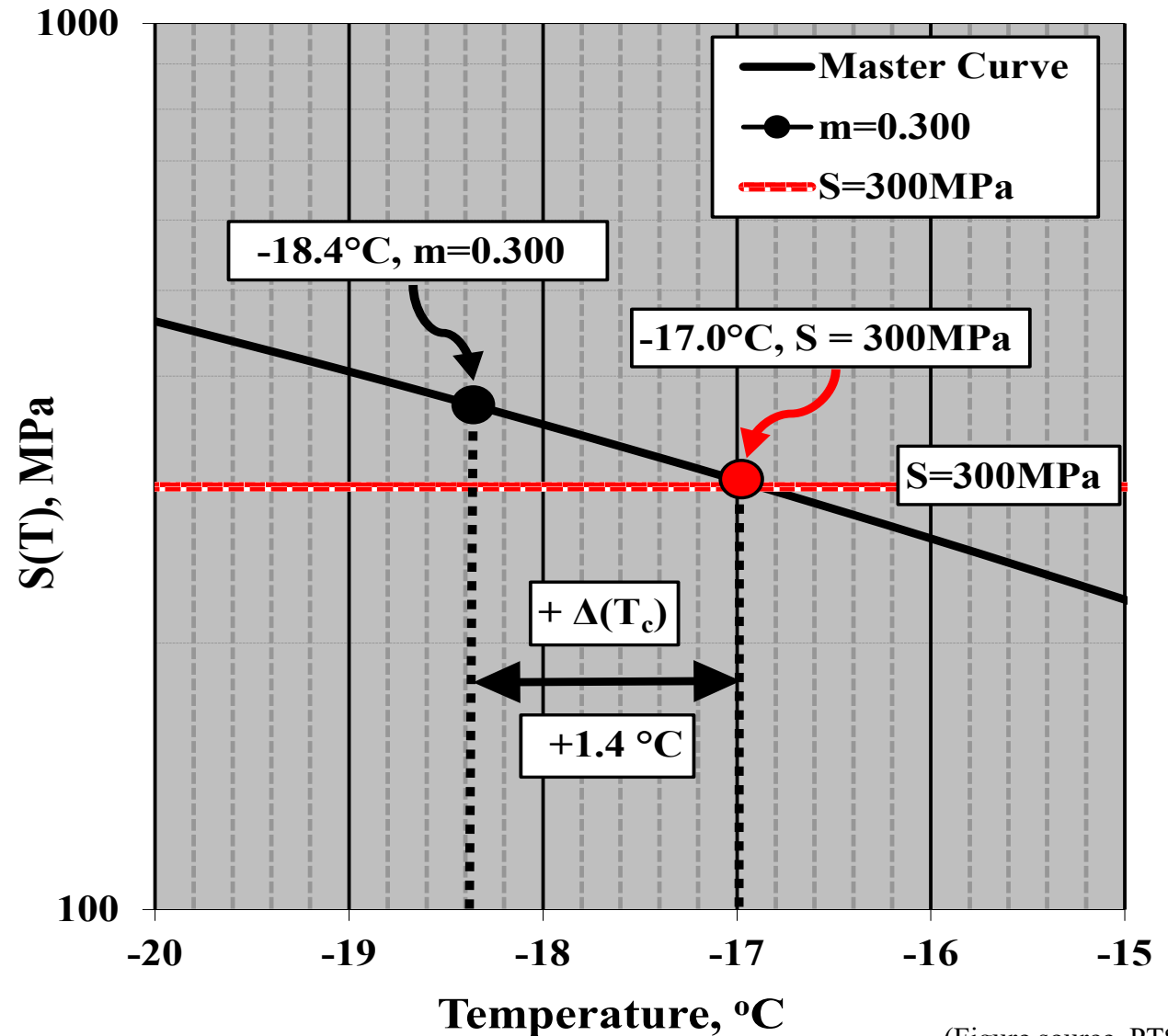
Determination of ΔT_c

Results of Bending Beam Rheometer Test are used to Determine ΔT_c

Critical Temperatures (T_c) are Calculated where AASHTO M 320 and AASHTO M 332 Limits for Creep Stiffness (S) and Creep Rate (m) meet $S=300$ Mpa and $m=0.300$

ΔT_c is Calculated by Subtracting the m -critical ($T_{c,m}$) Temp from the S -critical ($T_{c,S}$) Temp

$$\Delta T_c = T_{c,S} - T_{c,m}$$



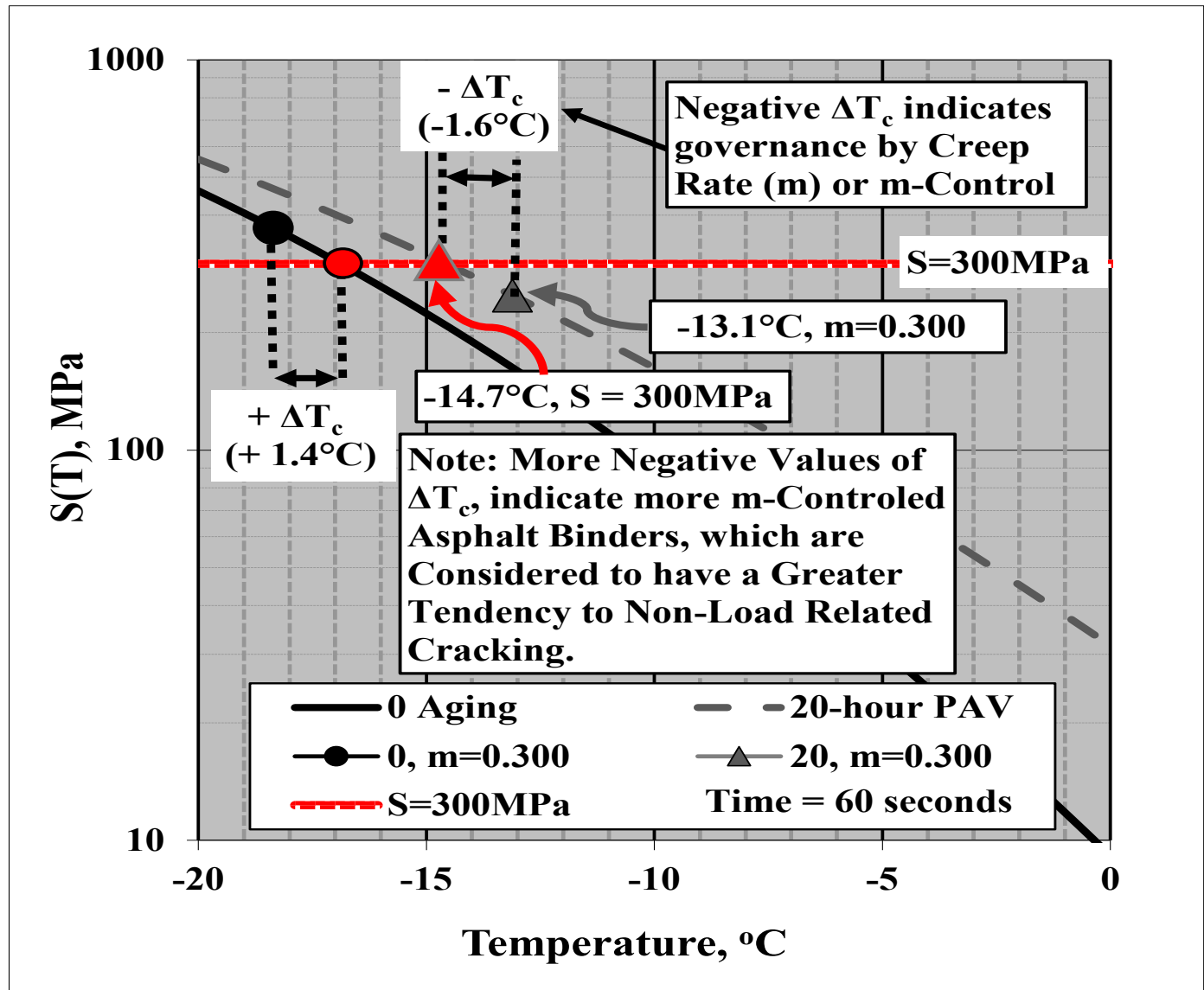
(Figure source, PTSi)

What Does it all Mean? (1 of 2)

A Positive Value of ΔT_c Indicates the Binder is “S-Controlled” (fails S before m).

A Negative Value of ΔT_c Indicates the Binder is “m-Controlled” (fails m before S).

The Magnitude of ΔT_c Indicates the Degree to Which the Binder is Either m-controlled or S-controlled.

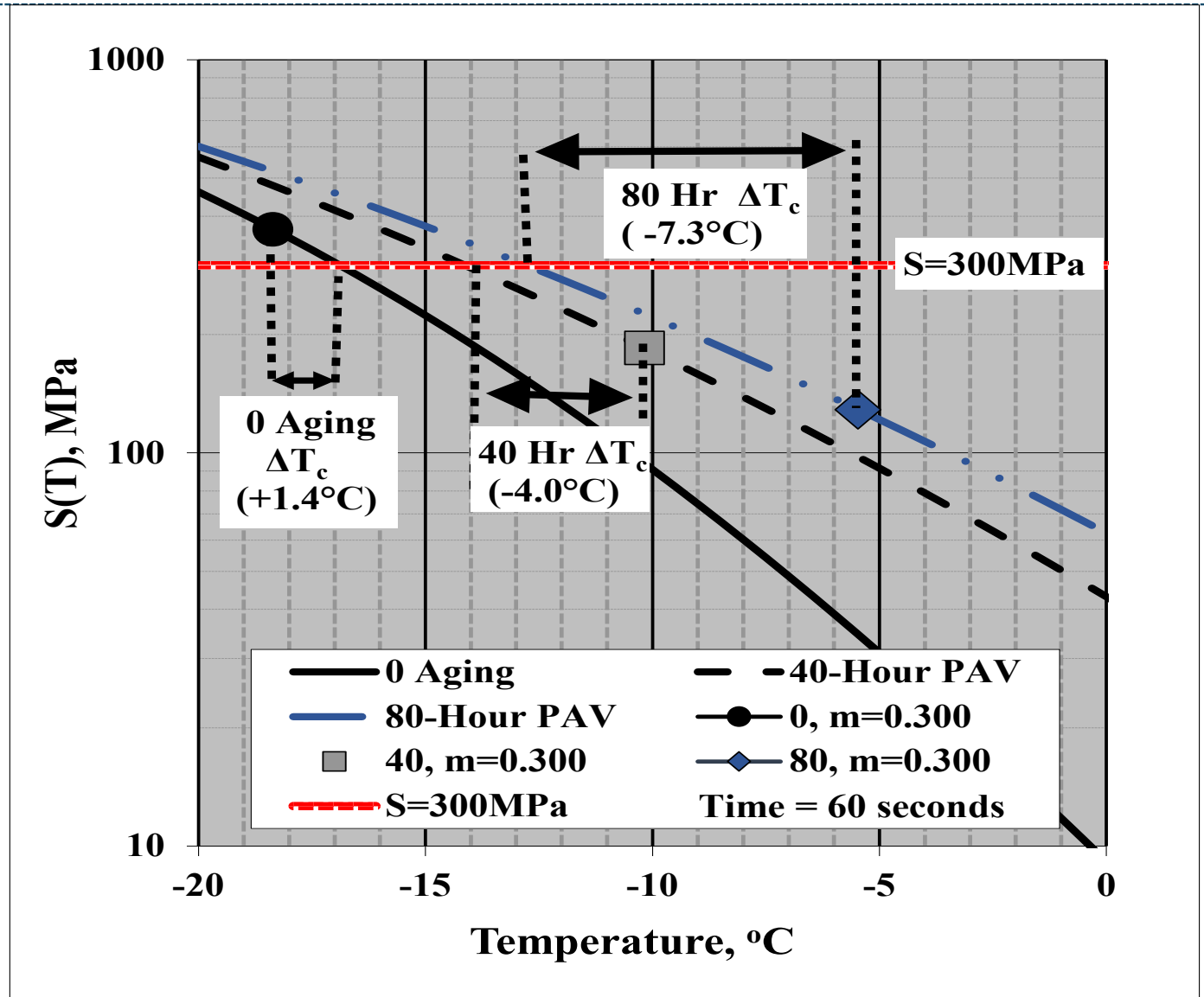


(Figure source, PTSi)

What Does it all Mean? (2 of 2)

The Magnitude of ΔT_c Indicates the Degree to Which the Binder is Either m-controlled or S-controlled.

More Negative Values of ΔT_c , more m-Controlled Asphalt Binders, are Considered to have a Greater Tendency to Non-Load Related Cracking



(Figure source, PTSi)

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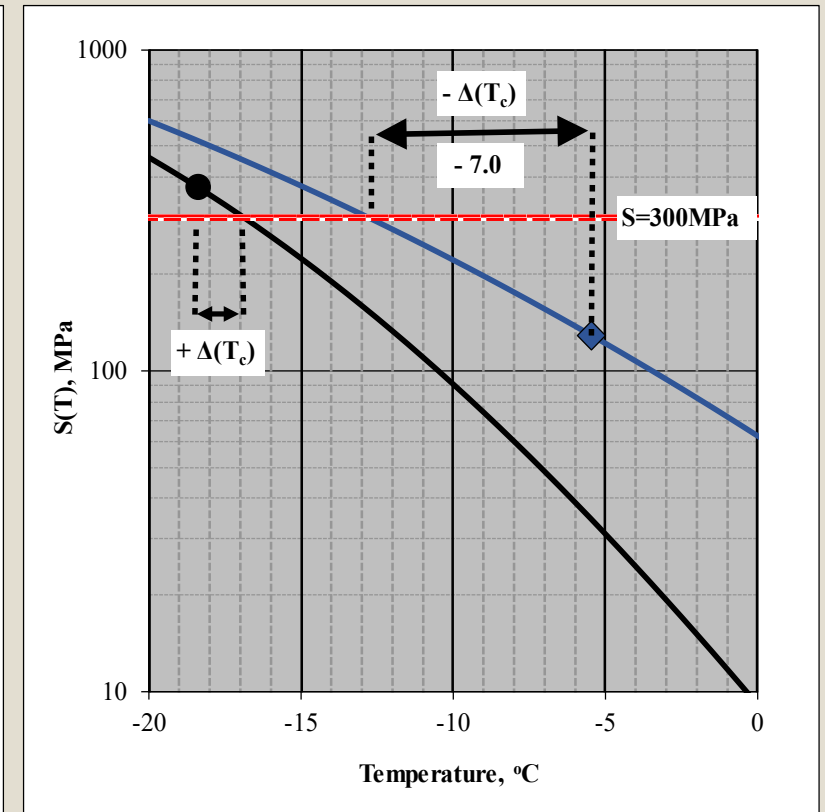
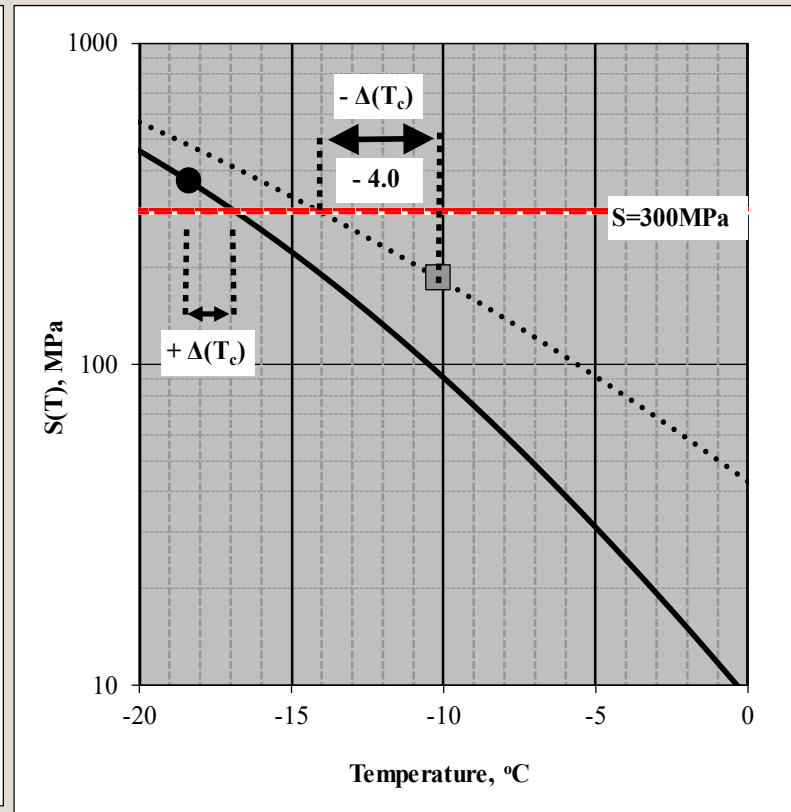
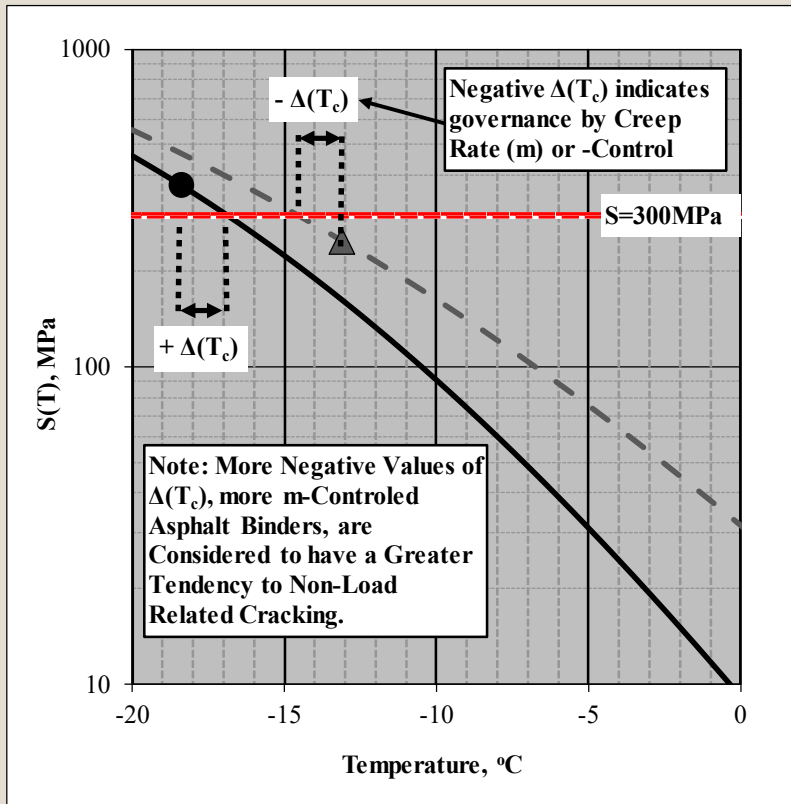
Asphalt Binder Aging Impact on ΔT_c

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- Asphalt Binder Response to **Aging** is the Primary Element Effecting ΔT_c .
- Laboratory **Aging** is Key to Evaluation of Asphalt Binder Durability and the Effect of ΔT_c on Pavement Durability.
 - As **Aging** Increases the Trend of ΔT_c is to become more Negative.
 - Extended PAV Aging Causes Asphalt Binders to become more m-Controlled (thus more negative values of ΔT_c)
- **How Much Laboratory Aging** is Needed to Adequately Evaluate ΔT_c .
- **No Simple Answer** to the Degree of Laboratory Aging Needed?

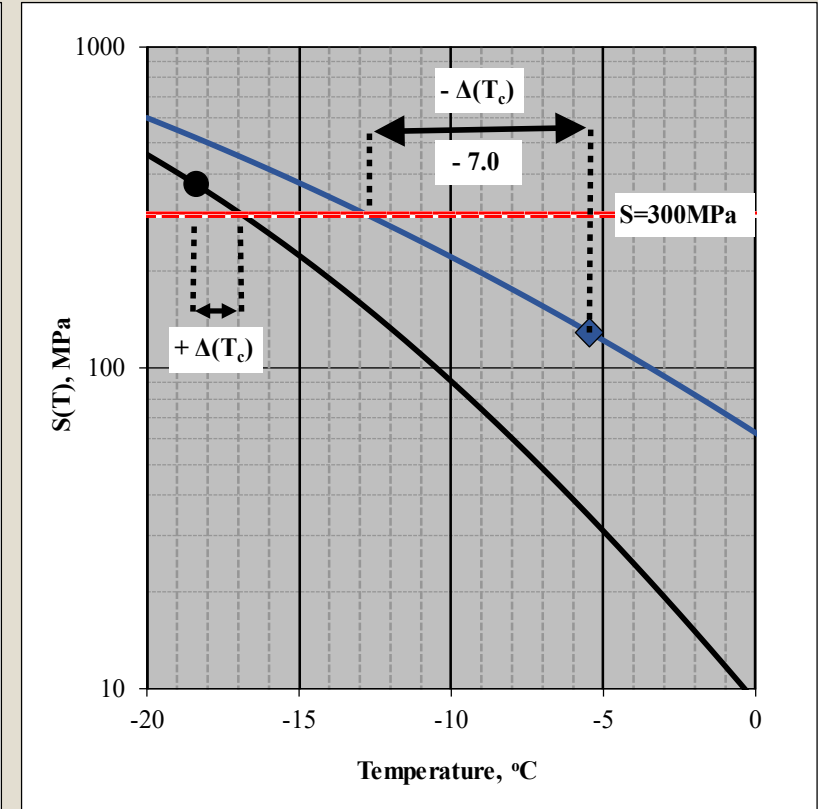
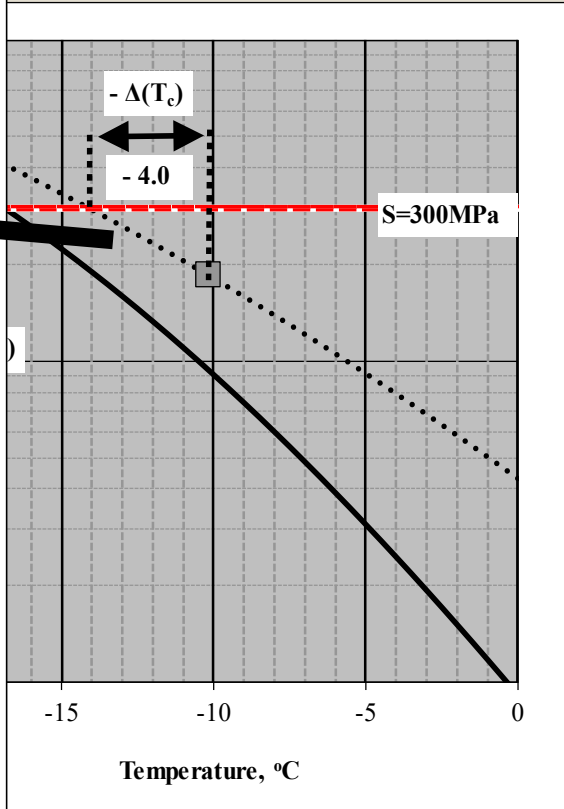
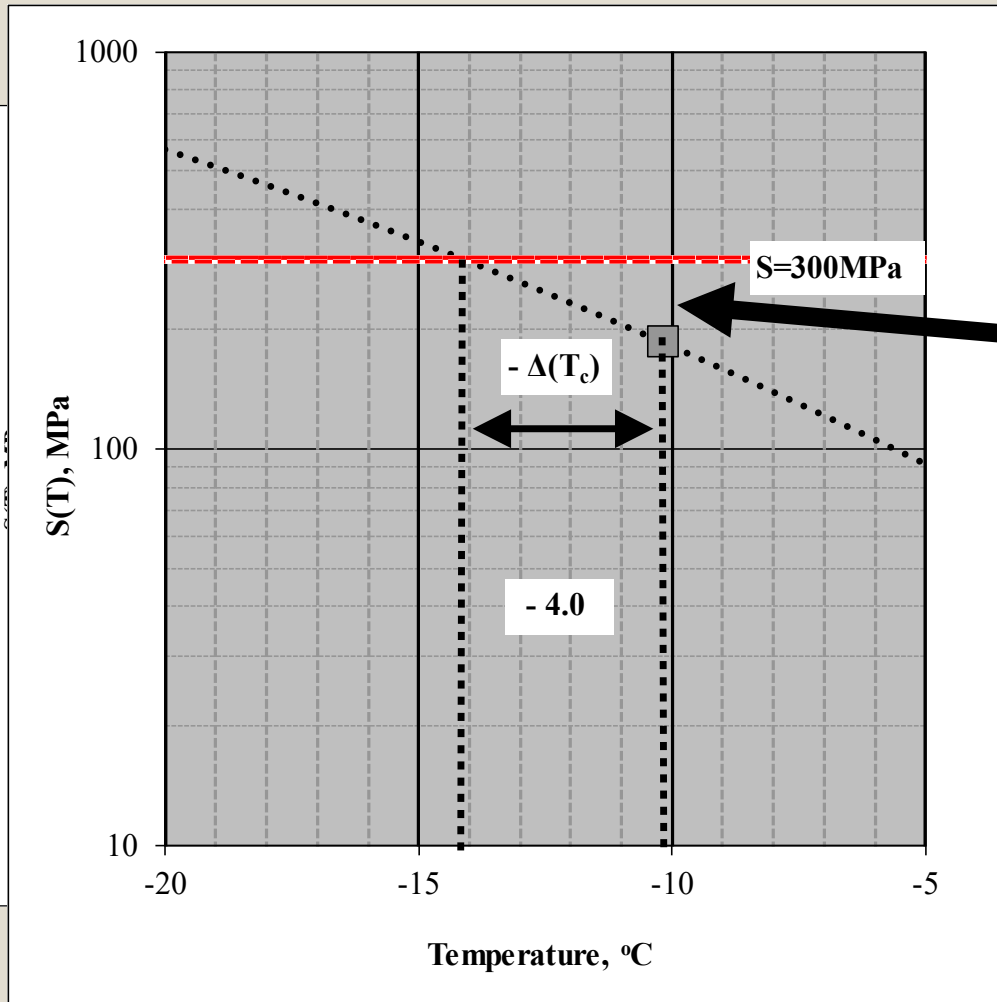
Laboratory Aging 20, 40, and 80 PAV Hours (1 of 2)

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Laboratory Aging 20, 40, and 80 PAV Hours (2 of 2)

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Elements Impacting ΔT_c

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- Asphalt Binder Response to **Aging, Aging, and Aging.**
- Effects of Additives on Asphalt Binder Properties and Aging Response
 - Reclaimed Asphalt Pavement (RAP).
 - Recycled Asphalt Shingles (RAS).
 - Re-refined Engine Oil Bottoms (REOB).
 - Elastomeric Polymer Modification.
 - Combined Effects.
- Air Rectified Asphalt Binders (Air Blown)

Impact of RAP on ΔT_c

As asphalt binder aging plays the primary role in ΔT_c performance, it is somewhat apparent the impact addition of age hardened Reclaimed Asphalt Pavement (RAP) binders will have on ΔT_c performance.

Effect of Recycled Asphalt Pavement on ΔT_c				
Asphalt Binder Blend	No RAP	10 Percent	20 Percent	40 Percent
PG52-34 Plus RAP A	2.2	0.2	0.1	0.7
PG64-22 Plus RAP A	-1.9	-2.7	-2.8	-4.4
PG52-34 Plus RAP B	2.2	0.4	-1.0	-2.8
PG64-22 Plus RAP B	-1.9	-3.4	-5.1	-4.8
PG52-34 Plus RAP C	2.2	-0.1	-0.7	-0.8
PG64-22 Plus RAP C	-1.9	-2.8	-3.1	-1.7

Data Source NCHRP Web Document 30 Project 09-12 October 2000

Impact of RAS on ΔT_c

Recycled Asphalt Shingles (RAS) asphalt binder is highly oxidized and very stiff. RAS, is expected to impact ΔT_c performance to a higher degree than RAP asphalt binder.

Calculation of the ΔT_c of RAS asphalt binder not as straight forward as with RAP asphalt binder due to difficulty of BBR analysis.

Estimated ΔT_c of Recycled Asphalt Shingle Binder

RAS Source	T_c High	T_c Low	ΔT_c
New Hampshire	163.0	12.0	-33.0
Oregon	152.0	14.0	-37.0
Texas	122.0	-7.0	-23.0
Wisconsin	146.0	16.0	-40.0
Wisconsin	146.0	6.0	-31.0

Data Source AI IS 240

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Implementation of the ΔT_c Parameter (1 of 2)

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- **Familiarize with ΔT_c .** AI IS-240 is a great starting point.
- ΔT_c parameter primarily intended to address durability related distresses.
- More negative values may have indirect impact on other forms of cracking.
- Clearly understand the performance challenge to be addressed.
- ΔT_c more than a number, it is not a panacea that cures all cracking issues.
 - Laboratory evaluation of existing pavements may be necessary.
 - **Alternative approaches to ΔT_c may prove more appropriate?**

Implementation of the ΔT_c Parameter (2 of 2)

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AI IS 240 suggests a five step systematic approach to implementation:

1. Clearly **identify the problem** ΔT_c is intended to address.
2. Determine whether ΔT_c is the **most favorable alternative**.
3. **Select aging method** to ensure ΔT_c measurements are representative.
4. **Evaluate existing pavements** that exhibit diverse cracking behavior.
5. **Evaluate ΔT_c results** obtained to **determine** simulative **aging protocol**.

Work together regionally to facilitate uniform transition for the asphalt industry.

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Alternatives to the ΔT_c Parameter (1 of 3)

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- Cracking - predominant distress affecting pavement durability.
- ΔT_c is just one of several alternatives available to address age-related embrittlement by specification means. Early on other alternatives were suggested as well:
 - Glover-Rowe (GR) Parameter
 - Rheological Index (R)
 - Cross-Over Modulus
 - Limiting (minimum) S-value

Alternatives to the ΔT_c Parameter (2 of 3)

- More recently, other alternatives have been proposed:
- Some research indicates that ΔT_c may be more effective at identifying deleterious affects of additives in asphalt binder than as a predictor of asphalt binder cracking or durability.
 - Propose minimum S-value for a given m-value.
 - Suggest variable S-value minimums applied to variable m-values for specific values of ΔT_c .
 - e.g., If $\Delta T_c = -8$ then the specification limit would be a minimum S-value of 125 MPa, with an allowable increase of the minimum S-value to 150 MPa for m-values greater than 0.32.

Alternatives to the ΔT_c Parameter (3 of 3)

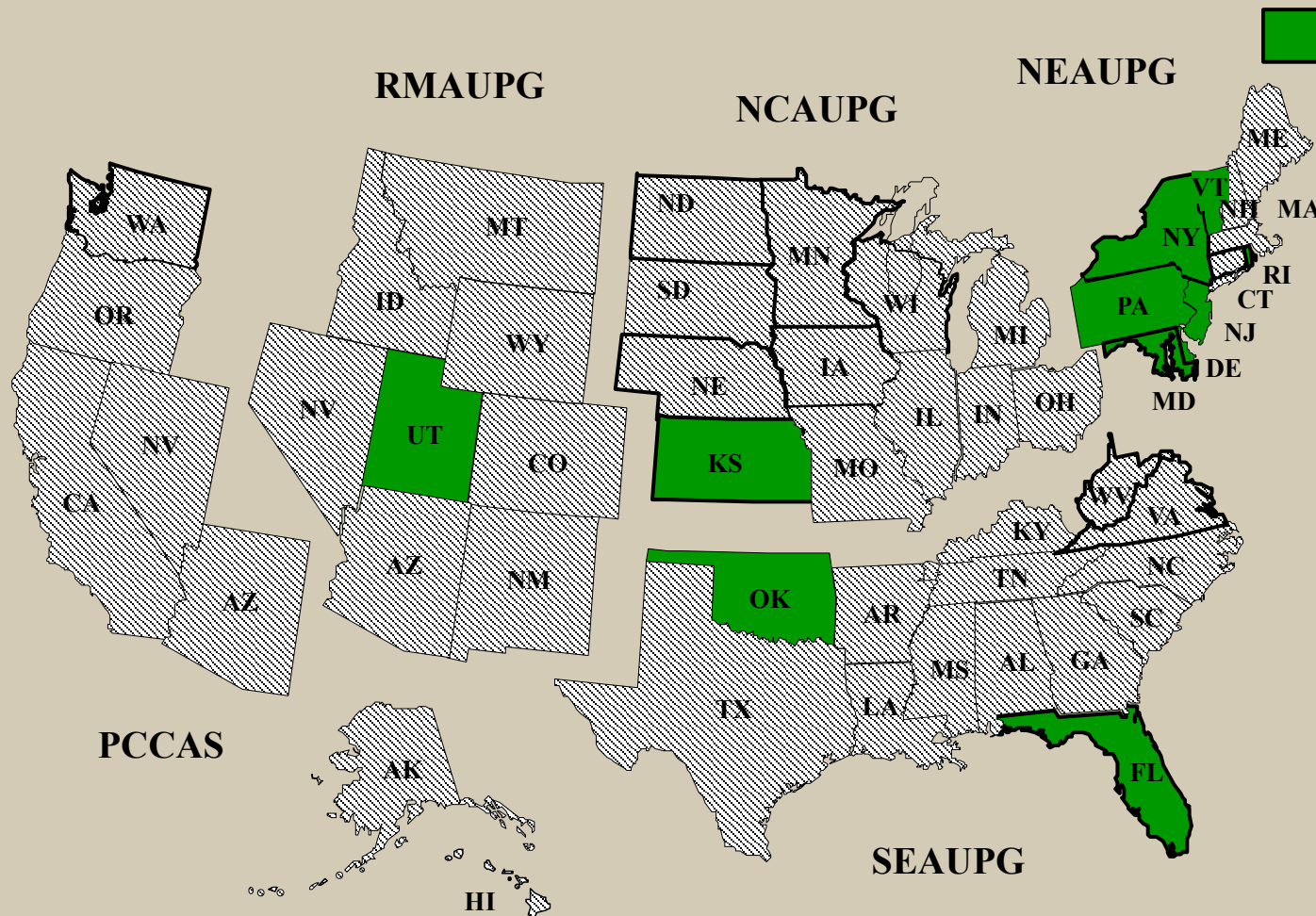
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- National Cooperative Highway Research Program (NCHRP) Project 9-60:
- A ΔT_f parameter would become an optional specification parameter, as was the case with the direct tension test. The ΔT_f parameter incorporates a binder fracture test using the Asphalt Binder Cracking Device (ABCD).
- Uses results from ΔT_c and ΔT_f after 20-hour PAV aging.
 - ΔT_c uses standard BBR results to calculate $T_{c,S} - T_{c,m} = \Delta T_c$.
 - ΔT_c threshold is set at -2 for warning and -6 for failure.
 - If, ΔT_c fails these limits then, ΔT_f is employed, where: $\Delta T_f = \text{ABCD } T_{c,f} - \text{BBR } T_{c,S}$,

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Status of Implementation of ΔT_c

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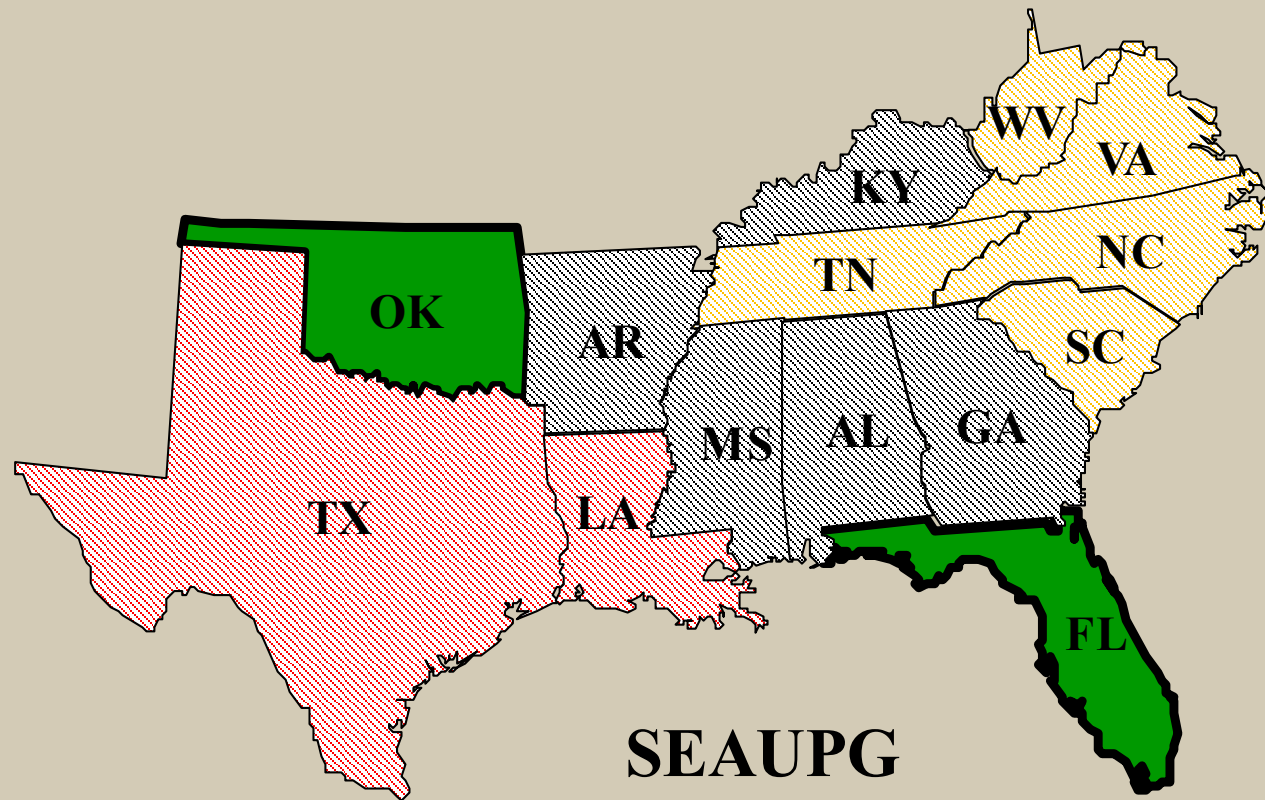


10 States Specifying ΔT_c

- DE, 40 Hr PAV $\Delta T_c \geq -5.0^\circ\text{C}$
- FL, 20 Hr PAV $\Delta T_c \geq -5.0^\circ\text{C}$
- KS, 40 Hr PAV $\Delta T_c \geq -5.0^\circ\text{C}$
- MD, 40 Hr PAV $\Delta T_c \geq -5.0^\circ\text{C}$
- NJ, 40 Hr PAV $\Delta T_c \geq -5.0^\circ\text{C}$
- NY, 40 Hr PAV $\Delta T_c \geq -5.0^\circ\text{C}$
- OK, 20 Hr PAV $\Delta T_c \geq -6.0^\circ\text{C}$
- PA, 40 Hr PAV $\Delta T_c \geq -5.0^\circ\text{C}$
- UT, 20 Hr PAV $\Delta T_c \geq -2.0^\circ\text{C}$
- VT, 40 Hr PAV $\Delta T_c \geq -5.0^\circ\text{C}$

Status of Implementation of ΔT_c in the SEAUPG Region

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- 2 States Specifying ΔT_c
 - FL, 20 Hr PAV $\Delta T_c \geq -5.0^\circ\text{C}$
 - OK, 20 Hr PAV $\Delta T_c \geq -6.0^\circ\text{C}$
- 2 States Looking at 4mm DSR
 - LA
 - TX, also looking at limiting BBR values
- 5 States monitoring or report only
 - NC, SC, TN, VA, WV
- 5 States not currently using ΔT_c
 - KY, AL, AR, GA, MS

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Summary (1 of 2)

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- Brief review of ΔT_c as a parameter to characterize asphalt binder behavior.

- Information Relies on AI IS-240:

Use of the Delta Tc Parameter to Characterize Asphalt Binder Behavior

- Objective is to provide knowledge to promote responsible deployment of ΔT_c as an asphalt binder purchase specification parameter.
- Presented a brief description of ΔT_c how it is determined, and relevance in characterizing the behavior of asphalt binders.

Summary (2 of 2)

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- Brief discussed elements impacting the ΔT_c parameter.
- Discussed possible steps to implementation of the ΔT_c parameter.
- Discussed possible alternatives to implementation of the ΔT_c parameter.
- Presented a brief overview of the current state of implementation of the ΔT_c parameter in the SEAUPG states.

Thank You.